KENTUCKY TRANSPORTATION CENTER

College of Engineering

RESULTS OF CREEP TESTS ON CONCRETE CYLINDERS FOR THE CABLE-STAYED BRIDGE AT OWENSBORO, KENTUCKY







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Report No. KTC-00-12

Results of Creep Tests on Concrete Cylinders for the Cable-Stayed Bridge at Owensboro, Kentucky

By

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in cooperation with

Kentucky Transportation Cabinet Commonwealth of Kentucky

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Introduction

The Kentucky Transportation Center was requested by the Kentucky Transportation Cabinet to conduct creep tests on the concrete from the new cable-stayed bridge over the Ohio River at Owensboro. The tests have been completed and the data is included herein.

Test Methods and Results

The creep tests were performed on cylinders according the ASTM C-512 (*Creep of Concrete in Compression*). In order to perform the creep tests the compressive strength of the cylinders must be determined. This was determined according to ASTM C-39 (*Compressive Strength of Cylindrical Concrete Specimens*). The modulus of elasticity was determined for one series according to ASTM C-469 (*Static Modulus of Elasticity and Poisson's Ratio of Concrete in Compression*). Three cylinders were loaded three days (3-day tests) after the cylinders were made, and three cylinders each were loaded at 30 days (30-day tests) and 90 days (90-day tests) after the cylinders were made. All of the tests ran for 180 days after loading.

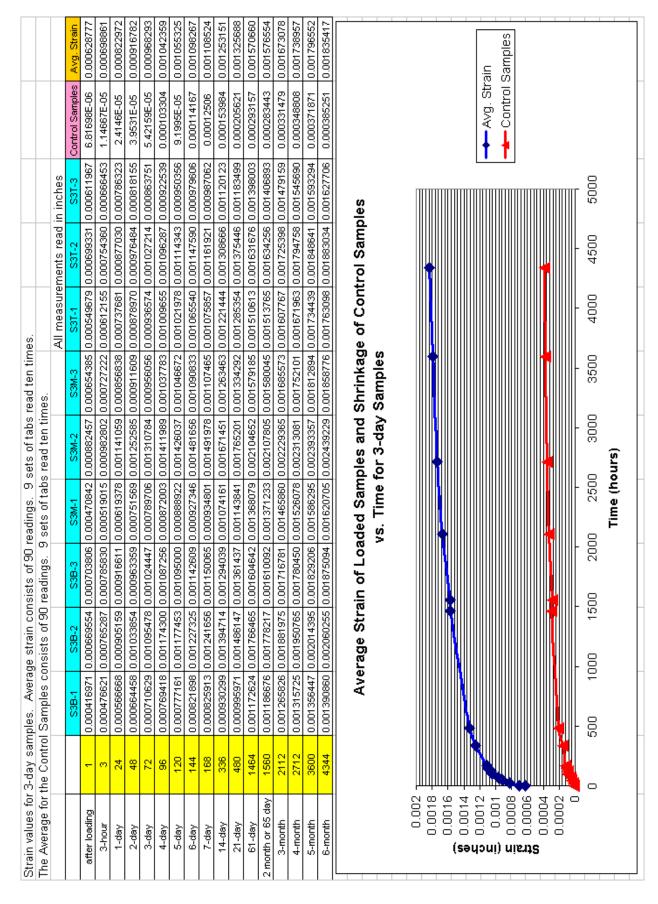
Results of the 3-day tests, 30-day tests, and 90-day tests are listed in Sections 1, 2 and 3, respectively. Section 4 lists the results of the modulus and Poisson's ratio tests for the cylinders at 3 days. Section 5 lists the dimensions and weight of the cylinders. Section 6 lists the ASTM standard test methods referenced above. No analysis was performed on the data as the purpose of the tests was only to provide baseline data for future reference, if necessary.



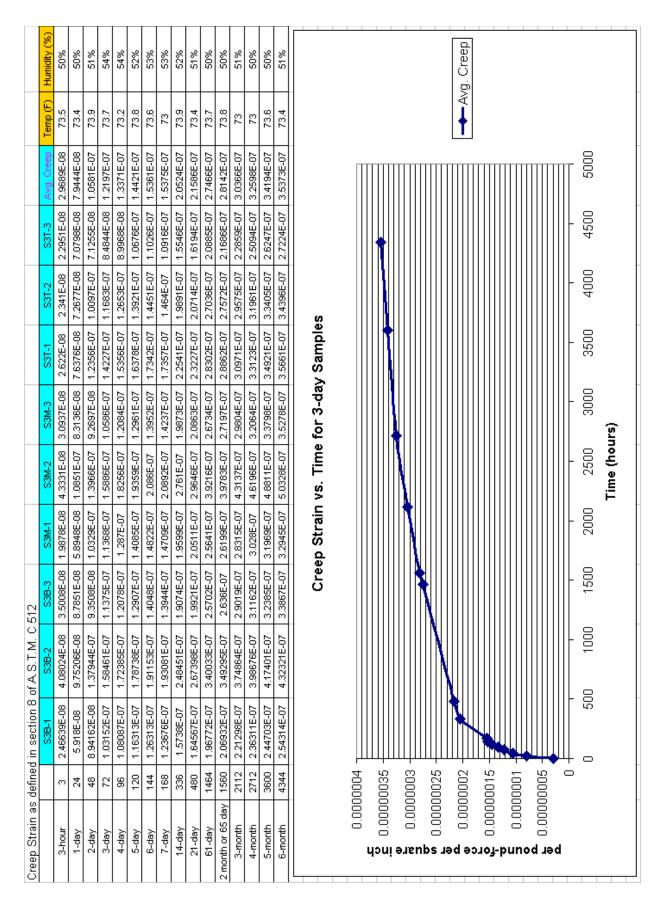
Virtual Image of Cable-Stayed Bridge, Owensboro

Section 1

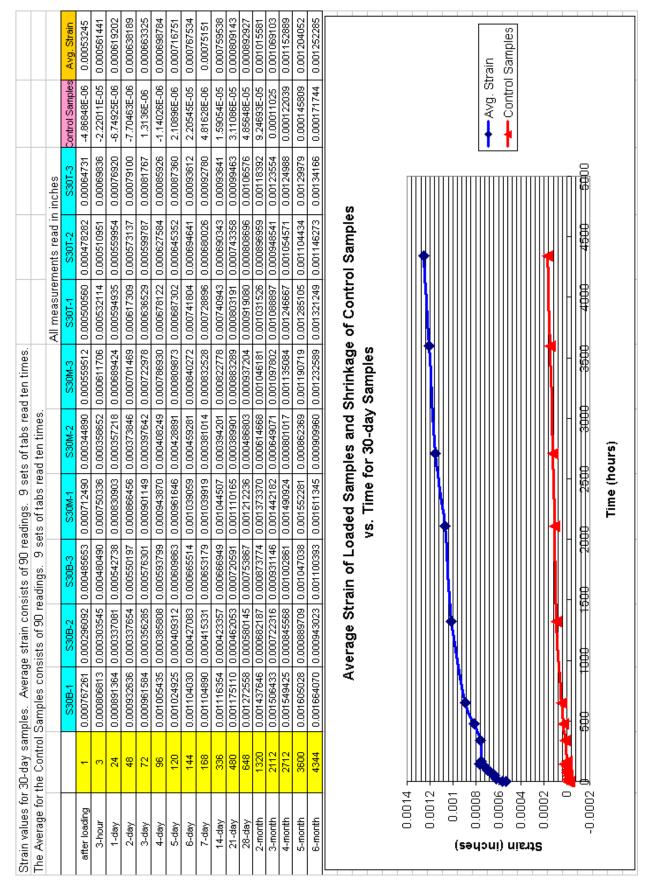
3-Day Creep Data



Cylinder 28.367 inches Cylinder 28.367 inches Cylinder 38.367 inches Cylinder 3 66035 234.097014 1 Sading 1 66495 234.097014 1 Second Average Stress Sading 1 66495 234.097014 1 Second 234.097	Total Load-Induced Strain per pound per square Inch	otrain per	bound	per square inc		Measureme	ents are defi	Measurements are defined as in section 8 of A.	ection 8 of A	. I.M.	212			
Average Load Average Stress S3B-1 S3B-2 S3B-3 S3	difference between	the average	strain	values of the	aded and	control spe	cimens divid	led by the av	verage stres	Š.				
Average Load Average Stress S8B-1 S3B-2 S3B-3 S3B-3 S3B-3 S3B-1 S3B-2 S3B-3 S3B-3 S3B-1 S3B-2 S3B-3 S3B-3 S3B-3 S3B-3 S3B-3 S3B-5 S3	Area of Cylinder	28	8.367 ir	nches										
Average Stress S38-1 S38-2 S38-3 S3M-1 S3M-2								Total Load-	Induced Str	ain per poun	d per square	e inch		
Average Load Average Stress S38-1 S38-2 S38-1 S38-1 S38-2 S38-1 S38-														Avg. Tot. Load Induced
1 66496 2344 097044 1.7497E-07 2.8736E-07 1.9795E-07 1.3795E-07 3.7355E-07 3.7355E-07 3.7355E-07 3.7355E-07 4.1688E-07 5.1321E-07 4.1688E-07 4.1688E-07 5.1321E-07 4.1688E-07 4.1688E-07 5.268E-07 5.268E-07 4.1688E-07 5.268E-07 5.268E-07 5.268E-07 5.268E-07 5.268E-07 5.268E-07 4.1688E-07 5.268E-07 5.268E-07 <t< td=""><td></td><td>Average</td><td></td><td>Average Stress</td><td>S3B-1</td><td>S3B-2</td><td>S3B-3</td><td>S3M-1</td><td>S3M-2</td><td>S3M-3</td><td>S3T-1</td><td>S3T-2</td><td>S3T.3</td><td>Strain</td></t<>		Average		Average Stress	S3B-1	S3B-2	S3B-3	S3M-1	S3M-2	S3M-3	S3T-1	S3T-2	S3T.3	Strain
3 66096 2323 996122 1,9964E-07 3,2355E-07 2,1783E-07 4,1688E-07 4,1688E-07 4,1688E-07 4,1688E-07 4,1688E-07 4,1688E-07 4,1688E-07 4,1688E-07 2,127E-07 <				2344.097014	1.7497E-07	2.8273E-07	2.9734E-07	1.9795E-07	3.7355E-07	2.7625E-07	2.3159E-07	2.9543E-07	2.582E-07	2.6533E-07
24 65726 2316.952797 2.3415E-07 3.8025E-07 3.8519E-07 4.5209E-07 4.5209E-07 4.5209E-07 4.5209E-07 4.5209E-07 4.5209E-07 4.5209E-07 4.5209E-07 5.1321E-07				2329.996122	1.9964E-07	3.2353E-07	3.3235E-07	2.1783E-07	4.1688E-07	3.0719E-07	2.5781E-07	3.1884E-07	2.811E-07	2.95019E-07
48 67050 2363.662002 26438E-07 3.0085E-07 3.0124E-07 5.1321E-07 72 66950 2360.136773 2.7813E-07 4.4119E-07 4.1109E-07 3.1163E-07 5.231E-07 10 66755 2352.26534 2.830E-07 4.4119E-07 4.1109E-07 3.1163E-07 5.501E-07 110 66755 2352.200027 2.9129E-07 4.614E-07 4.1109E-07 3.501E-07 5.611E-07 114 66630 2346.74031 2.9129E-07 4.5181E-07 3.4618E-07 5.611E-07 3.4618E-07 5.612E-07 166 66030 2346.74037 3.3335E-07 4.3781E-07 4.3618E-07 5.612E-07 5.436E-07 5.612E-07 1464 67110 2346.74037 3.3335E-07 4.3618E-07 4.3618E-07 5.631E-07 5.631E-07 4.5439E-07 5.631E-07 5.631E-07 5.631E-07 5.631E-07 4.3608E-07 5.631E-07 5.631E-07 5.631E-07 5.631E-07 5.631E-07 5.631E-07 5.631E-07 5.631E-07 5.631E-07 5.6				2316.952797	2.3415E-07	3.8025E-07	3.8519E-07	2.569E-07	4.8206E-07	3.5939E-07	3.0796E-07	3.6811E-07	3.29E-07	3.44774E-07
72 66950 2360136779 2.7813E-07 4.4119E-07 4.1109E-07 3.1163E-07 5.3241E-07 96 66755 2353.262594 2.8306E-07 4.511E-07 4.1812E-07 3.266E-07 5.5612E-07 120 66725 2352.205072 2.9123E-07 4.6148E-07 4.781E-07 3.266E-07 5.6712E-07 144 66635 2246.740331 2.986E-07 4.738E-07 3.4618E-07 5.812E-07 168 66200 2336.740724 3.3356E-07 4.738E-07 3.4618E-07 5.814E-07 168 66030 2327.74173 3.7175E-07 4.2369E-07 4.505E-07 5.824E-07 1750 67090 2365.072091 3.8191E-07 6.276E-07 5.438E-07 7.030E-07 7.030E-07 7.538E-07 7.030E-07 7.039E-07 7.030E-07 7.039E-07 7.039E-07 7.039E-07 7.039E-07 7.039E-07 7.039E-				2363.662002	2.6439E-07	4.2067E-07	3.9085E-07	3.0124E-07	5.1321E-07	3.6895E-07	3.5514E-07	3.964E-07	3.294E-07	3.71141E-07
36 66755 2353.262594 2,8306E-07 4,5511E-07 4,1812E-07 3,2665E-07 5,5612E-07 120 66725 2352.205027 2,3123E-07 4,514E-07 3,2665E-07 5,5612E-07 144 66635 2349.03236 3,0123E-07 4,7581E-07 3,460E-07 5,814E-07 336 66570 2346.740931 2,386E-07 4,7581E-07 3,460E-07 5,814E-07 336 66570 2335.81274 3,2335E-07 4,7581E-07 4,806E-07 5,804E-07 4460 66030 2327.70472 3,235E-07 5,305E-07 4,5030E-07 6,706E-07 2112 66890 2357.845384 3,9627E-07 6,5738E-07 4,811E-07 8,894E-07 7,713E-07 2712 66890 2357.845384 4,1128E-07 6,5738E-07 4,811E-07 8,894E-07 7,713E-07 3600 66550 2346.035887 4,1968E-07 7,7013E-07 6,2118E-07 8,718E-07 8,718E-07 8,718E-07 3600000 66550 2342.510684			20	2360.136779	2.7813E-07	4.4119E-07	4.1109E-07	3.1163E-07	5.3241E-07	3.8211E-07	3.7386E-07	4.1226E-07	3.43E-07	3.87299E-07
120 66725 2352 205027 2 129E-07 4.6146E-07 4.2641E-07 3.388E-07 5.6715E-07 144 66635 2349.032326 3.0129E-07 4.7388E-07 3.4618E-07 5.6715E-07 168 66570 2346.740931 2.9865E-07 4.7381E-07 3.4618E-07 5.8215E-07 336 66200 2335.81274 3.235E-07 5.318E-07 4.8665E-07 4.0565E-07 5.8045E-07 5.804E-07 5.804E-07 5				2353.262594	2.8306E-07	4.5511E-07	4.1812E-07	3.2665E-07	5.5612E-07	3.971E-07	3.8515E-07	4.2196E-07	3.481E-07	3.99044E-07
144 66635 2349,032326 3.0128E-07 4.7388E-07 4.3781E-07 3.4618E-07 5.8215E-07 168 66570 2346,740931 2.9865E-07 4.7581E-07 4.3678E-07 5.8145E-07 5.8215E-07 336 66260 2335,81274 3.3235E-07 5.3118E-07 4.8608E-07 3.3934E-07 5.845E-07 6.4965E-07 1464 67110 2385,777135 3.7175E-07 5.3012E-07 4.8608E-07 7.691E-07 6.4508E-07 6.4965E-07 2112 66886 2357,845384 3.8131E-07 6.3205E-07 5.8365E-07 7.138E-07 2712 66886 2357,845384 3.8131E-07 6.5758E-07 5.0076E-07 7.138E-07 3600 68550 2346,035887 4.1986E-07 7.0013E-07 6.2119E-07 5.0776E-07 8.6168E-07 3000000 4344 66450 2342,510684 4.2929E-07 7.1505E-07 5.2741E-07 8.6168E-07 3000000 4300 4344 66450 2342,510684 4.2929E-07 7.1505E-07				2352.205027	2.9129E-07	4.6146E-07	4.2641E-07	3.388E-07	5.6715E-07	4.0586E-07	3.9537E-07	4.3463E-07	3.649E-07	4.09543E-07
168 66570 2346.740931 2.9865E-07 4.7581E-07 4.3679E-07 5.8247E-07 5.8247E-07 336 66280 2335.81274 3.3235E-07 5.3118E-07 4.8808E-07 3.3934E-07 6.4965E-07 1464 67110 2365.770472 3.335E-07 5.5012E-07 5.503E-07 6.4965E-07 1560 67090 2365.072091 3.8191E-07 6.2278E-07 5.638E-07 7.7138E-07 2112 66885 2357.845384 3.967E-07 6.5758E-07 5.0076E-07 7.7138E-07 210 66850 2346.035887 4.1968E-07 6.2198E-07 5.0076E-07 8.6450 3434 66450 2342.510684 4.2329E-07 7.1505E-07 5.2741E-07 8.6450 3000000 4.334 66450 2342.510684 4.2329E-07 7.1505E-07 5.2741E-07 8.686E-07 3000000 4.334 66450 2342.510684 4.2329E-07 7.1505E-07 5.2741E-07 8.686E-07 30000000 4.336 4.336 4.336			35	2349.032326	3.0129E-07	4.7388E-07	4.3781E-07	3.4618E-07	5.8215E-07	4.1577E-07	4.0501E-07	4.3994E-07	3.684E-07	4.18938E-07
336 66260 2335,81274 3.3235-07 5.318E-07 4.8808E-07 3.3394E-07 6.4965E-07 480 66030 2327,704727 3.3354E-07 5.5012E-07 4.3808E-07 6.7001E-07 1464 67110 2367,77435 3.7775E-07 5.5012E-07 5.503E-07 6.7001E-07 1560 67090 236,072091 3.819TE-07 6.207E-07 5.503E-07 7.738E-07 2112 68680 2350,371199 4.1128E-07 6.3758E-07 5.803E-07 7.7138E-07 3600 68550 2346,035887 4.128E-07 6.314E-07 6.308E-07 8.186E-07 3600 2346,035887 4.128E-07 6.318E-07 5.2741E-07 8.186E-07 3600 2342,510664 4.2929E-07 7.1505E-07 6.36E-07 8.7638E-07 3600 3600 342,510664 4.2929E-07 7.1505E-07 6.36E-07 8.7638E-07 3600 3600 342,510664 4.2929E-07 7.1505E-07 6.3741E-07 8.7638E-07 3600			20	2346.740931	2.9865E-07	4.7581E-07	4.3678E-07	3.4505E-07	5.8247E-07	4.1863E-07	4.0516E-07	4.4183E-07	3.673E-07	4.19077E-07
480 66030 2327.704727 3.3954E-07 5.5012E-07 4.3655E-07 4.0307F-07 6.7001E-07 1464 67110 2365.777135 3.775E-07 6.2276E-07 5.5436E-07 7.6571E-07 1560 67090 2365.07291 3.8191E-07 6.3202E-07 5.693E-07 7.738E-07 2112 66890 2350.971199 4.1128E-07 6.3769E-07 5.8753E-07 4.811E-07 8.0492E-07 3600 66550 236.035887 4.128E-07 6.319E-07 5.076E-07 8.168E-07 3600 66550 2346.035887 4.128E-07 7.0013E-07 5.2741E-07 8.168E-07 3600 66550 2342.510664 4.2929E-07 7.1505E-07 5.2741E-07 8.7635E-07 3600 66550 2342.510664 4.2929E-07 7.1505E-07 5.2741E-07 8.7635E-07 3600 66550 2342.510664 4.2929E-07 7.1505E-07 5.2741E-07 8.7635E-07 3600 6650 6650 6650 6650 7.2741E-07 <			90	2335.81274	3.3235E-07	5.3118E-07	4.8808E-07	3.9394E-07	6.4965E-07	4.7499E-07	4.57E-07	4.9434E-07	4.136E-07	4.70571E-07
1464 67110 2365.777135 3.7175E-07 6.2276E-07 5.5436E-07 4.5436E-07 7.6571E-07 1560 67090 2365.072091 3.8191E-07 6.3202E-07 5.6093E-07 4.5394E-07 7.7138E-07 2112 66886 2357.845384 3.3627E-07 6.3759E-07 4.8111E-07 8.0492E-07 2712 66890 2350.371199 4.1128E-07 6.814E-07 5.0076E-07 8.0492E-07 3600 66850 2346.035887 4.1968E-07 7.0013E-07 6.2119E-07 8.166E-07 4344 66450 2342.510664 4.2929E-07 7.1505E-07 6.2119E-07 8.7683E-07 100000 56650 2342.510664 4.2929E-07 7.1505E-07 6.2119E-07 5.2741E-07 8.6836E-07 100000 56690 2342.510664 4.2929E-07 7.1505E-07 5.2741E-07 8.7683E-07 100000 56690 5676E-07 5676E-07 5676E-07 5676E-07 5676E-07 5676E-07 5676E-07 56766E-07 5676E-07 5676E-07				2327.704727	3.3954E-07	5.5012E-07	4.9655E-07	4.0307E-07	6.7001E-07	4.8489E-07	4.6386E-07	5.0257E-07	4.201E-07	4.81189E-07
1560 67090 2365.072091 3.8191E-07 6.3202E-07 5.6093E-07 4.5994E-07 7.7138E-07 2112 66885 2357.845384 3.9627E-07 6.5759E-07 5.8753E-07 4.8111E-07 8.0492E-07 2712 66890 2350.371199 4.1128E-07 6.814E-07 5.0076E-07 8.0492E-07 3600 66850 2346.035887 4.1968E-07 7.0013E-07 6.2119E-07 5.7765E-07 8.6166E-07 100000 66850 2342.510664 4.2929E-07 7.1505E-07 6.2119E-07 5.7741E-07 8.6166E-07 1000000 1000000 100000				2365.777135	3.7175E-07	6.2276E-07	5.5436E-07	4.5436E-07	7.6571E-07	5.436E-07	5.1461E-07	5.6578E-07	4.67E-07	5.39993E-07
2112 66885 2357.845384 3.9627E-07 6.5759E-07 4.8111E-07 8.0492E-07 2712 66890 2350.971199 4.1128E-07 6.814E-07 6.0896E-07 5.0076E-07 8.3552E-07 3600 66550 2346.035887 4.1988E-07 7.0013E-07 6.2119E-07 5.076E-07 8.3552E-07 4344 66450 2342.510664 4.2829E-07 7.1505E-07 6.2119E-07 5.741E-07 8.7683E-07 0.00000007 Total Load-Induced Strain vs. Time for 3-day Strain vs.				2365.072091	3.8191E-07	6.3202E-07	5.6093E-07	4.5994E-07	7.7138E-07	5.4823E-07	5.2021E-07	5.7115E-07	4.75E-07	5.46753E-07
2712 66690 2350371199 4.1128E-07 6.814E-07 6.0896E-07 5.0076E-07 8.3552E-07 3600 66550 2346.035887 4.1968E-07 7.0013E-07 6.2119E-07 5.1765E-07 8.6166E-07 10.0000000 66550 2342.510664 4.2929E-07 7.1505E-07 6.2119E-07 5.1765E-07 8.6166E-07 10.0000000 7.00100000 7.1505E-07 6.2119E-07 5.2741E-07 8.7683E-07 10.0000000 7.00100000 7.1505E-07 6.2119E-07 7.2741E-07 8.7683E-07 10.0000000 7.00100000 7.00100000 7.00100000 7.00100000 10.0000000 7.00100000 7.00100000 7.00100000 7.00100000				2357.845384	3.9627E-07	6.5759E-07	5.8753E-07	4.8111E-07	8.0492E-07	5.7429E-07	5.4129E-07	5.9118E-07	4.867E-07	5.68994E-07
3600 66550 2346.035887 4.1968E-07 5.2119E-07 5.1765E-07 8.6166E-07 7.0010E-07 5.2141E-07 8.765E-07 8.6166E-07 7.1505E-07 7.1505E-07 7.1505E-07 7.1505E-07 7.1505E-07 7.1505E-07 7.1505E-07 7.1505E-07 8.765E-07 8.765E-0				2350.971199	4.1128E-07	6.814E-07	6.0896E-07	5.0076E-07	8.3552E-07	5.969E-07	5.6281E-07	6.1504E-07	5.091E-07	5.91308E-07
4344 66450 2342.510664 4.2929E-07 7.1505E-07 5.2741E-07 8.7683E-07 5.00000007			20	2346.035887	4.1968E-07	7.0013E-07	6.2119E-07	5.1765E-07	8.6166E-07	6.1424E-07	5.808E-07	6.2947E-07	5.206E-07	6.07271E-07
0.0000007 0.0000006 0.0000003 0.0000001		_	99	2342.510664	4.2929E-07	7.1505E-07	6.36E-07	5.2741E-07	8.7683E-07	6.2904E-07	5.8819E-07	6.3939E-07	5.304E-07	6.19065E-07
0			- 009		otal Loax	Y-Inducec Y-Inducec Time (h	4 Strain v:	S. Time fc	or 3-day S	samples 5		- Avg. Tot. Load Induced Strain	oad Induce	ed Strain

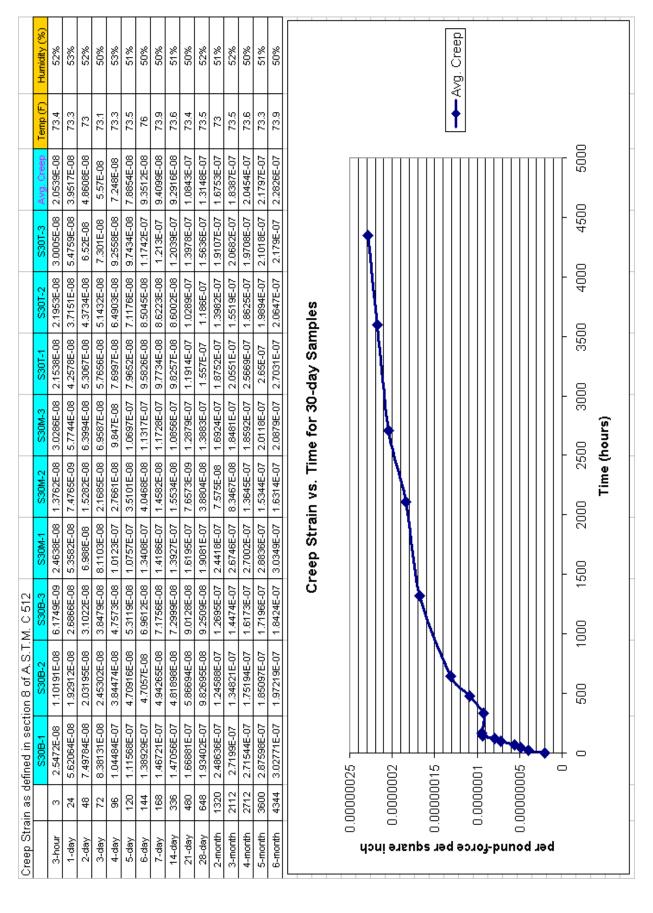


Section 2 30-Day Creep Data

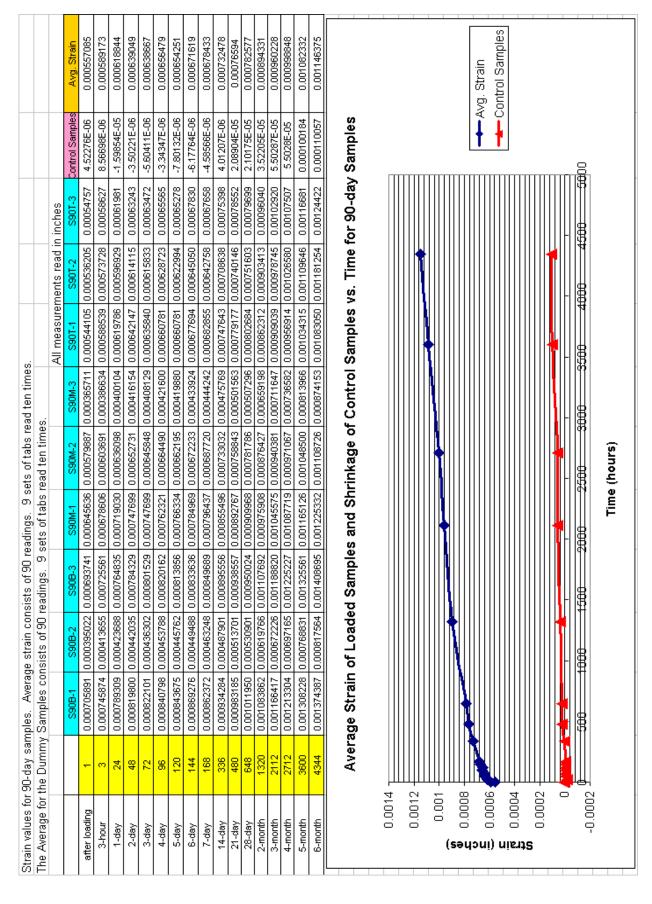


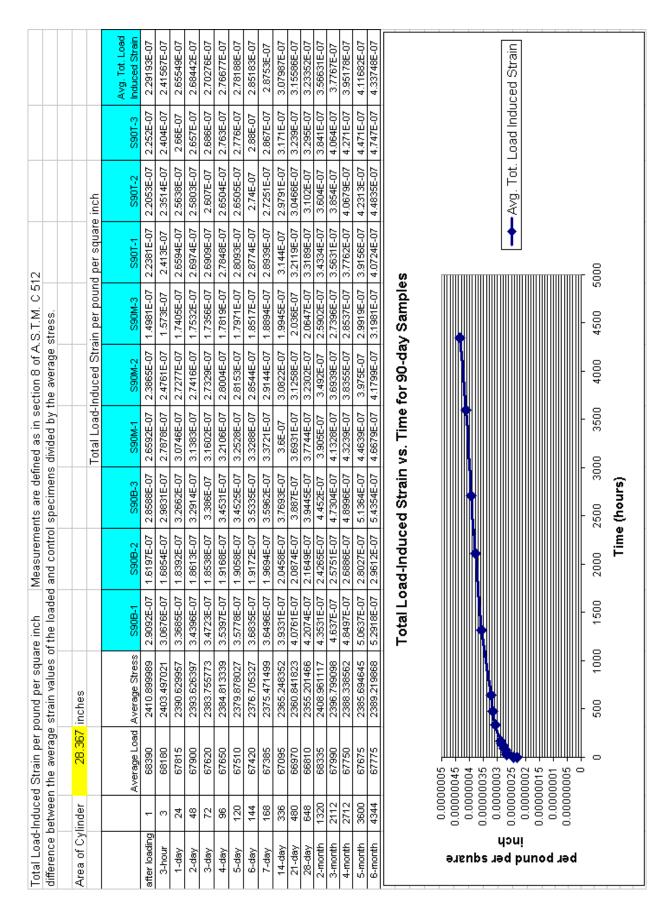
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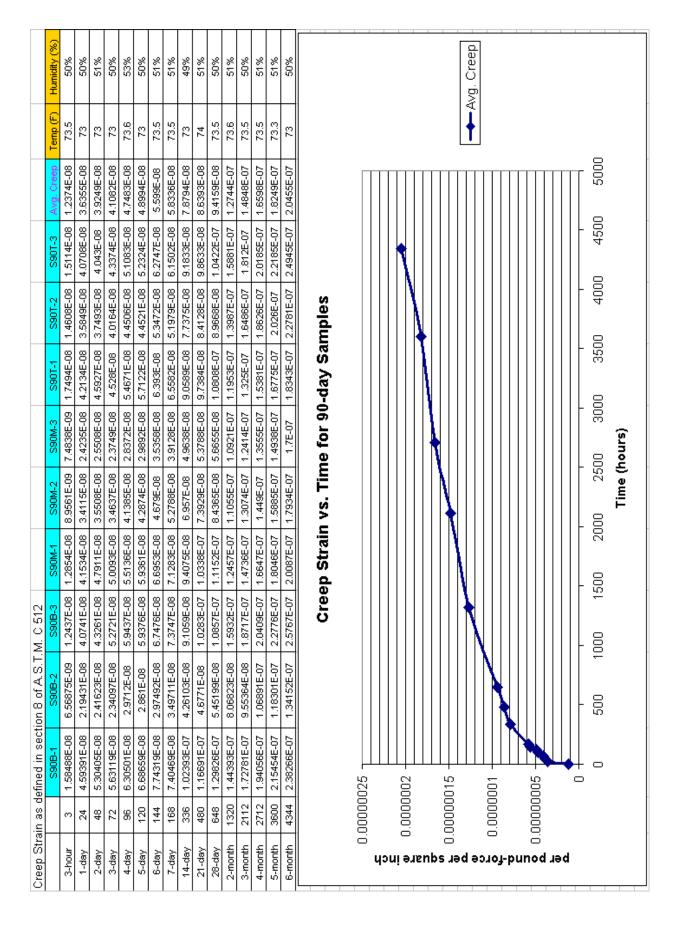
lotal Load-	Induce	d Strain per k	lotal Load-Induced Strain per pound per square inc	2 S.L.	Measureme	Measurements are defined as in section 6 of A.	ned as In se	ction 8 of A	اد	212			
difference b	oetwee	n the average	difference between the average strain values of the	_	and control	loaded and control specimens divided by the average stress	divided by t	the average	stress.				
Area of Cylinder	inder	28.367	28.367 inches										
							Total Load-I	Induced Stra	Total Load-Induced Strain per pound per square inch	d per square	e inch		
													Avg. Tot. Load
		Average Load	Average Load Average Stress	S30B-1	S30B-2	S30B-3	S30M-1	S30M-2	S30M-3	S30T-1	S30T-2	S30T-3	Induced Strain
after loading	1	68310	2408.079811	3.2064E-07	1.2498E-07	2.037E-07	2.979E-07	1.4524E-07	2.3437E-07	2.0989E-07	2.0064E-07	2.708E-07	2.23132E-07
3-hour	3	67945	2395.212747	3.4611E-07	1.36E-07	2.0987E-07	3.2253E-07	1.5901E-07	2.6466E-07	2.3143E-07	2.2259E-07	3.008E-07	2.4367E-07
1-day	24	67605	2383.226989	3.7685E-07	1.4427E-07	2.3056E-07	3.5148E-07	1.5272E-07	2.9211E-07	2.5247E-07	2.3779E-07	3.256E-07	2.62649E-07
2-day	48	67425	2376.881588	3.9562E-07	1.453E-07	2.3472E-07	3.6778E-07	1.6053E-07	2.9836E-07	2.6296E-07	2.4437E-07	3.36E-07	2.7174E-07
3-day	72	67350	2374.237671	4.0445E-07	1.4951E-07	2.4218E-07	3.79E-07	1.6693E-07	3.0396E-07	2.6754E-07	2.5207E-07	3.438E-07	2.78831E-07
4-day	96	67165	2367.716008	4.2513E-07	1.6343E-07	2.5127E-07	3.9912E-07	1.729E-07	3.3284E-07	2.8689E-07	2.6554E-07	3.634E-07	2.95612E-07
5-day	120	67130	2366.48218	4.3221E-07	1.7207E-07	2.5682E-07	4.0547E-07	1.8034E-07	3.4134E-07	2.8954E-07	2.7181E-07	3.683E-07	3.01985E-07
6-day	144	66785	2354.320161	4.5957E-07	1.7204E-07	2.7331E-07	4.3197E-07	1.8571E-07	3.4754E-07	3.0571E-07	2.8568E-07	3.883E-07	3.16643E-07
7-day	168	66770	2353.791377	4.6736E-07	1.7441E-07	2.7545E-07	4.3976E-07	1.5983E-07	3.5165E-07	3.0762E-07	2.8686E-07	3.921E-07	3.1723E-07
14-day	336	66745	2352.910072	4.677E-07	1.7317E-07	2.767E-07	4.3716E-07	1.6078E-07	3.4293E-07	3.0815E-07	2.8664E-07	3.912E-07	3.16048E-07
21-day	480	99599	2346.56467	4.8752E-07	1.8365E-07	2.9383E-07	4.5985E-07	1.529E-07	3.6316E-07	3.2903E-07	3.0353E-07	4.106E-07	3.31563E-07
28-day	648	67545	2381.111855	5.1404E-07	2.2325E-07	2.9621E-07	4.8871E-07	1.8405E-07	3.732E-07	3.6559E-07	3.1923E-07	4.272E-07	3.54608E-07
2-month	1320	67030	2362.956957	5.6928E-07	2.4957E-07	3.3065E-07	5.4208E-07	2.2099E-07	4.0361E-07	3.9741E-07	3.4046E-07	4.619E-07	3.9066E-07
3-month	2112	66830	2355.906511	5.9263E-07	2.598E-07	3.4844E-07	5.6536E-07	2.2871E-07	4.1918E-07	4.154E-07	3.5583E-07	4.776E-07	4.07E-07
4-month	2712	68375	2410.371206	5.9219E-07	3.0017E-07	3.6543E-07	5.6791E-07	2.8169E-07	4.2029E-07	4.6658E-07	3.8688E-07	4.679E-07	4.27672E-07
5-month	3600	68055	2399.090492	6.0824E-07	3.1008E-07	3.7565E-07	5.8625E-07	2.9868E-07	4.3554E-07	4.7489E-07	3.9958E-07	4.81E-07	4.41102E-07
6-month	4344	67905	2393.802658	6.2341E-07	3.222E-07	3.8794E-07	6.0139E-07	3.0839E-07	4.4316E-07	4.802E-07	4.0711E-07	4.887E-07	4.51391E-07
per pound per square	0.000.000.000.0000.0000.0000.0000.0000.0000	0.00000005 0.000000045 0.00000003 0.00000003 0.000000015 0.000000015 0.000000015	- 1000		ad-Induce	Total Load-Induced Strain vs. Time for 30-day Samples	vs. Time	for 30-da	y Sample	<u>\$</u>	-Avg. Tot.	Load Indu	Tot. Load Induced Strain



Section 3 90-Day Creep Data







Section 4 Modulus of Elasticity

Static Modulus		of Elasticity for 3 Day Creep Samples	3 Day Creep	Samples					
DATE	5/27/1999								
Compressi	Compressive Strength for 3 day breaks	3 day breaks							
	A	В	0	Avg.		Load Rate 60000 lbs per minute	00 lbs per m	inute	
Max Load		173400	176000	172033.3333		Vertical Gage Length = 16 inches	Length = 16	inches	
X-Sect(A)		28.2744	28.2744			Horizontal Gage Length = 12 inches	te Length = 1	12 inches	
Comp Str.	5895.79266	6132.756133	6224.712107	6084.4203					
Static Moc	- 1 - T- 1								
40% of	172033.333 <mark>3</mark>	<u>.</u>	68813.33333 lbs.	ps.					
Sample 3-A	Ą								
Load	Vert. 1	Vert. 2	Avg. Vert.	Long. Strain	Stress(psi)	Horizont. 1	Horizont. 2	Horizont. 2 Avg. Horizont	Transverse Strain
0	0	0	0	0	0	0	0		
			0	0	0				0
8600		0.0008		0.00005	304.1620689		0		0
8000	0.0008			0.00005	282.9414594	0			0
10000	0.001	0.001	0.001	0.0000625	353.6768243	0	0	0	0
15000	0.0015	0.0015	0.0015	0.00009375	530.5152364	0.0001	0	0.00005	4.16667E-06
20000	0.0021	0.002	0.00205	0.000128125	707.3536485	0.0002	0.00015	0.000175	1.45833E-05
25000	0.0028	0.0027	0.00275	0.000171875	884.1920607	0.0003	0.0002	0.00025	2.08333E-05
30000	0.0033	0.0033	0.0033	0.00020625	1061.030473	0.0004	0.0003	0.00035	2.91667E-05
35000	0.00395	0.0039	0.003925	0.000245313	1237.868885	0.0005	0.0004	0.00045	0.0000375
40000	0.0046	0.0045	0.00455	0.000284375	1414.707297	9000:0	0.0005	0.00055	4.58333E-05
45000	0.005	0.0051	0.00505	0.000315625	1591.545709	0.0008	9000:0	0.0007	5.83333E-05
20000	0.0057	0.0057	0.0057	0.00035625	1768.384121	0.0009	0.0007	0.0008	6.66667E-05
22000	0.0064	0.0064	0.0064	0.0004	1945.222533	0.001	0.0008	0.0009	0.000075
00009	0.007	0.007	0.007	0.0004375	2122.060946	0.0011	0.00095	0.001025	8.54167E-05
65000	0.0077	0.0077	0.0077	0.00048125	2298.899358	0.0012	0.00105	0.001125	0.00009375
00889	0.0082			0.0005125	2433.296551	0.0013			0.000108333
00889		0.008		0.0005	2433.296551		0.0011		9.16667E-05
/00V +0 00000 - CO									
02 – SIIBS	is at 40 % of ult.	Luau S 0.3							
67 - Lurigi	ez = Longitudinal strain (& SZ 61 = chross @ chroin 5670 vir	g 0.2			ווחצי	L	ACADAAD A		
SITES	SI = stress @ strain of 50 uln/in	U UIIIVIII				Ш ;	4649416.4	lsd	
= - Young's Moduli	E = Tourigs Modulus of Elasticity	Hasticity					0.2342342		
040 - 0000	Words official of	of 3 transverse etrain at mid-height of the energineer	0000		0.00				
Drod	produced by etrace S2	C)	10000			Ш	4731410 nei		
041 = 47000	worce etrain at	off = transverse effects at mid-height of the energines	0000			 =	0.2037037	5	
LI LI LI LI LI LI LI LI	overse strain at		d shacillall			l	0.2007.007		
prod	produced by stress of	VI 00000 00000	10000						
	al Strain at JUUC	Longitudinal Strain at .UUUUSUUUUUSU " 16 = .UUUG F = 7000 047777-0 00000007447	0000 = 91						
0.7S)) = I	E = ((52-51)/((62-0.000050	UUUUSUJJ"A)							
19-719) = N	1)/(ez-u.uuuusu								

Sample 3-C	C Vort 1	C #0\	Ave Vor	Long Ofrain	Ctrocolnoil	Horizont 1	Horizont 2	2 Ava Horizont	Transverse Otrain
	- _		المراجعة المراجعة	Colligi Citiani	0(1633(þ31)			1110711011:BAC	IIIaiioydiod Oilaiii
									0
8600		0.0008		0.00005	304.1620689		0.0001		8.33333E-06
8000	0.0008			0.00005	282.9414594	0.0001			8.33333E-06
10000	0.001	0.001	0.001	0.0000625	353.6768243	0.0002	0.0001	0.00015	0.0000125
15000	0.0016	0.0016	0.0016	0.0001	530.5152364	0.0003	0.0002	0.00025	2.08333E-05
20000	0.0022	0.0021	0.00215	0.000134375	707.3536485	0.00035	0.00035	0.00035	2.91667E-05
25000	0.0029	0.0029	0.0029	0.00018125	884.1920607	0.00045	0.00045	0.00045	0.0000375
30000	0.0036	0.0036	0.0036	0.000225	1061.030473	95000.0	0.00055	95000.0	4.58333E-05
35000	0.0043	0.0044	0.00435	0.000271875	1237.868885	0.00065	0.00065	0.00065	5.41667E-05
40000	0.005	0.0052	0.0051	0.00031875	1414.707297	0.0008	0.0008	9000:0	6.66667E-05
45000	0.0057	0.0059	0.0058	0.0003625	1591,545709	6000'0	0.0009	6000'0	0.000075
20000	0.0065	0.0066	0.00655	0.000409375	1768.384121	0.001	0.001	0.001	8.33333E-05
55000	0.0071	0.0073	0.0072	0.00045	1945.222533	0.0011	0.00105	0.001075	8.95833E-05
90009	0.0079	0.0079	0.0079	0.00049375	2122.060946	0.0012	0.00115	0.001175	9.79167E-05
65000	0.0085	9800.0	0.00855	0.000534375	2298.899358	0.00135	0.0013	0.001325	0.000110417
00889	0.009			0.0005625	2433.296551	0.00145			0.000120833
00889		0.0091		0.00056875	2433.296551		0.0014		0.000116667
S2 = stress	S2 = stress at 40% of ult.	Load							
e2 = Longit	e2 = Longitudinal strain @	S2			Run 1				
S1 = stress	S1 = stress @ strain of 50	ı uin/in				E=	4195814.8	isd	
E = Young	E = Young's Modulus of El	lasticity				= 0	0.2195122		
u = Poisson's ratio	n's ratio								
et2 = trans	et2 = transverse strain at mid-height of the specimen	nid-height of the	e specimen		Run 2				
produ	produced by stress S2	32				Ш	4104355.6	isd	
et1 = trans	et1 = transverse strain at mid-height of the specimen	nid-height of the	e specimen			= -	0.2088353		
produ	produced by stress S1	21							
Longitudina	Longitudinal Strain at .000050000050 * 16 =	. 050000° - 050	. 16 = .0008						
E = ((S2-S))	$E = ((S2-S1)/((e2-0.000050))^*A)$	J))*A)							
u = (et2-et1	u = (et2-et1)/(e2-0.000050)								
						ШΙ		31	
				ЗА	run 1	4649416.414 psi	psi	0.23423423	
					run 2	4731409.96	psi	0.2037037	
				30	run1	4195814.813		0.2195122	
					run2	4104355.628	isd	0.20883534	
				Otopodova Docios	20.00	315053 1606		0.012/0122	
				Granualu Davia		4420240.1036		0.04010.0	
				Average		4420249.204			
				Coemcient of Variation	ariation	7.146064964			

Section 5

Cylinder Height, Diameter, and Weight

unit weigl	nt of concrete for (Owensboro Bridg	je			
Date						
cylinder	weight(lbs)	height(in)	diameter(in)	area(in^2)	volume(ft^3)	unit weight (lbs/cf)
1	28.3	11.977	5.9935	28.21310601	0.195548826	144.7208896
2	29.1	11.965	6.006	28.33091082	0.196168604	148.3417804
3	29.2	12.0195	5.99	28.18016464	0.196013593	148.9692604
4	29	11.981	6.014	28.40643471	0.196954568	147.2420784
5	28.7	12.022	6.006	28.33091082	0.197103131	145.6090519
6	29.2	12.001	5.997	28.24606662	0.196169586	148.8508006
7	29	11.995	5.993	28.20839892	0.195810038	148.1027242
8	28.7	11.9915	5.9935	28.21310601	0.195785568	146.5889461
9	28.8	12.011	5.9995	28.26962169	0.196496774	146.5672915
10	28.4	12.0025	5.997	28.24606662	0.196194106	144.7546036
	average (Diam.)		5.99895	inches		
	average (area)		28.26443873	inches^2		
average u	init weight	146.9747427	lbs/cf			

Section 6

ASTM C-39 ASTM C-469 ASTM C-512

C39/C39M-01 Standard Test Method for Compressive Strength of Cylindrical Concrete Specimens

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1. Scope

- 1.1 This test method covers determination of compressive strength of cylindrical concrete specimens such as molded cylinders and drilled cores. It is limited to concrete having a unit weight in excess of 50 lb/ft³ [800 kg/m³].
- 1.2 The values stated in either inch-pound or SI units are to be regarded separately as standard. The SI units are shown in brackets. The values stated in each system may not be exact equivalents; therefore, each system shall be used independently of the other. Combining values from the two systems may result in nonconformance with the standard.
- 1.4 This standard does not purport to address all of the safety concerns, if any, associated with its use. It is the responsibility of the user of this standard to establish appropriate safety and health practices and determine the applicability of regulatory limitations prior to use.
- 1.4 The text of this standard references notes which provide explanatory material. These notes shall not be considered as requirements of the standard.

C469-94e1 Standard Test Method for Static Modulus of Elasticity and Poisson's Ratio of Concrete in Compression

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1. Scope

- 1.1 This test method covers determination of (1) chord modulus of elasticity (Young's) and (2) Poisson's ratio of molded concrete cylinders and diamond-drilled concrete cores when under longitudinal compressive stress. Chord modulus of elasticity and Poisson's ratio are defined in Terminology E 6.
- 1.2 The values stated in inch-pound units are to be regarded as the standard.
- 1.3 This standard does not purport to address all of the safety concerns, if any, associated with its use. It is the responsibility of the user of this standard to establish appropriate safety and health practices and determine the applicability of regulatory limitations prior to use.

C512-87(1994) Standard Test Method for Creep of Concrete in Compression

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1. Scope

- 1.1 This test method covers the determination of the creep of molded concrete cylinders subjected to sustained longitudinal compressive load. This test method is limited to concrete in which the maximum aggregate size does not exceed 2 in. (50 mm).
- 1.2 The values stated in inch-pound units are to be regarded as the standard.
- 1.3 This standard does not purport to address all of the safety problems, if any, associated with its use. It is the responsibility of the user of this standard to establish appropriate safety and health practices and determine the applicability of regulatory limitations prior to use.